SLCC BUSINESS BUILDING 175 South Main Street, Suite 300 Salt Lake City, Utah 84111 CHILLER REPLACEMENT **CONSULTANTS** HFS ARCHITECTS 329 SOUTH RIO GRANDE STREET Salt Lake Community College SALT LAKE CITY, TUAH 84101 FAX: 1-801-596-0691 TEL: 1-801-596-0691 EMAIL: bsmith@hfsa.com Redwood Campus THOMAS & KOLKMAN
ELECTRICAL ENGINEERS
64 WEST 1700 SOUTH
SALT LAKE CITY, UTAH 84115
TEL: 1-801-484-8161 FAX: 1-801-484-8161 EMAIL: grandyb@utahmicro.com State of Utah-Department of Administrative Services AND MANAGEMENT 4110 State Office Building/Salt Lake City, Utah 84114/538-3018 **GENERAL ABBREVIATIONS DFCM PLAN CHECK CODE ANALYSIS** MATERIALS LEGEND **GENERAL NOTES DRAWING INDEX** STRUCTURAL DRAWINGS APPLICABLE CODES VERIFIED PRIOR TO COMMENCING WORK - ANY DISCREPANCIES ARE TO BE MAINT. MAINTENANCE SG-101 GENERAL STRUCTURAL NOTES National Electrical Code _____2005 International Building Code REPORTED TO THE ARCHITECT OR ENGINEER OF RECORD PRIOR TO COMMENCING SLCC SE-101 FOUNDATION PLAN, FOOR FRAMING PLAN & DETAILS International Plumbing Code Building Conservation ARCH. ARCHITECTURAL M.O. MASONRY OPENING ADA Accessibility REDWOOD ROAD International Fire Code MAX. MAXIMUM International Energy PROTECT ALL AREAS & SURFACES ADJACENT TO DEMOLITION & CONSTRUCTION. Conservation Code PATCH & REPAIR ANY DAMAGE OR HOLES RESULTING FROM THE DEMOLTION OF Accessibility CAMPUS EXISTING ITEMS OR THE CONSTRUCTION OF NEW ITEMS. A. Occupancy and Group: STATE PROPERTY NO: Change in Use: Yes N/A No Mixed Occupancy: Yes N/A No No MIRROR . NOTED AREAS INDICATED THE GENERAL EXTENT OF DEMOLITON. THE ARCHITECTURAL DRAWINGS Special Use and Occupancy (e.g. High Rise, Covered Mall): N/A MISC. MISCELLANEOUS Energy G-100 TITLE SHEET, GENERAL INFO & DRAWING INDEX MORE OR LESS DEMOLITION. THE MEANS & METHODS OF DEMOLITION & B. Seismic Design Category: Design Wind Speed: 90 mph SLCC BUSINESS CONSTRUCTION MUST BE ACCOUNTED FOR IN THE CONTRACTORS BID. ANY DEMOLITION PLAN, NEW FLOOR PLAN & ROOF PLAN C.I. CAST IRON DEMOLITION BEYOND THE AREAS INDICATED IN THE CONTRACT DOCUMENTS WILL C. Type of Construction (circle one): BUILDING ELEVATIONS, BUILDING SECTIONS & WALL SECTIONS NORTH BUILDING NOT BE COMPENSATED FOR AFTER THE BID OPENING. N.I.C. NOT IN CONTRACT Plumbing Specification A-801 EXISTING CONDITION PHOTOGRAPHS CHILLER $_{
m H.}$ $\,$ PRIOR TO THE START OF DEMOLTION, THE CONTRACTOR IS TO MEET WITH THE D. Fire Resistance Rating Requirements for the Exterior Walls based on the fire OWNER & ARCHITECT TO IDENTIFY ALL ITEMS TO BE DEMOLISHED, REMOVED & separation distance (in hours): REPLACEMENT RETURNED TO THE OWNER, AND REMOVED & REINSTALLED. APPROVAL DOES NOT RELIEVE A/E OF DESIGN LIABILITY North: 0 South: 0 East: 0 West: 0 MECHANICAL DRAWINGS E. Mixed Occupancies: N/A Nonseparated Uses: N/A 5. 72-HOUR NOTICE IS REQUIRED FOR ANY UTILITY SHUT DOWN. OPP. OPPOSITE M-001 MECHANICAL SYMBOL LEGEND & GENERAL NOTES OPP. H. OPPOSITE HAND ALTERNATE #1 BUSINESS BUILDING PARTIAL PLOT PLAN O.D. OUTSIDE DIAMETER MECHANICAL ROOM PLAN Required: NO Provided: NO Type of Sprinkler System: N/A DFCM PRE-BID REVIEW MECHANICAL DETAILS & SCHEDULES G: Number of Stories: __1 __ Building Height: __15'-4" SALT LAKE CITY, UTAH PIPING SCHEMATIC H: Actual Area per Floor (square feet): 624 PART. PARTITION PED. PEDESTRIAN PLAS. PLASTER ELECTRICAL DRAWINGS J: Area Modifications: PL PLATE PLYWD. PLYWOOD BUSINESS BUILDING ELECTRICAL DEMOLITION PLAN - ADD ALT. #1 CHILLER BUILDING LIGHTING AND FIRE ALARM PLANS PROJECT MANUAL: CHILLER BUILDING POWER PLAN SYMBOL LIST, SCHEDULES AND DIAGRAMS R.B. RUBBER BASE E-602 CONTROL WIRING DIAGRAMS R.W.L. RAIN WATER LEADER b) Sum of the Ratio Calculations for Mixed Occupancies: R.F.F. REFERENCE FINISH FLOOR RISER c) Total Allowable Area for: OTHER AGENCIES R.D. ROOF DRAIN 1) One Story: <u>8,500</u> RM. ROOM 2) Two Story: A_a(2)_____ R.O. ROUGH OPENING METAL 3) Three Story: A_a(3)_____ REVIEW DOES NOT RELIEVE A/E OF DESIGN LIABILITY d) Unlimited Area Building: Yes N/A No Code Section: K. Fire Resistance Rating Requirements for Building Elements (hours). FLOOR DRAIN SL./SLP. SLOPE FACE OF STUD Listing Floors - Ceiling Floors F.O.W. FACE OF WALL S.C. SOLID CORE Exterior Bearing Walls FTG. FOOTING PROJECT DIRECTORY DFCM APPROVALS **GRAPHIC SYMBOLS** FOUNDATION VICINITY MAP Exterior Doors and Windows FINISH FLOOR Shaft Enclosures Structural Frame MARK DATE DESCRIPTION Partitions - Permanent Fire Walls Fire Partitions STOR. STORAGE STRUCT STRUCTURAL/STRUCTURE ARCHITECTURAL CONSULTANT DATE: RM. NAME/NUMB **APPROVALS:** DFCM PROJECT NO: 05238660 RM. FINISH SYMB. L. Design Occupant Load: 2 GYP. BD. GYPSUM BOARD 329 South Rio Grande TEMP. TEMPORARY / TEMPERED PROJECT NO: 20050650 GWB GYPSUM WATERPROOF BOARD Exit Width Required: ___0.4"_ THK. THICK (NESS) Salt Lake City, Utah 84101 DRAWN BY: 801-596-0691/Fax 801-596-0693 CHECKED BY: CEIL. FINISH/ELEV. SYMB. T/CURB TOP OF CURB STRUCTURAL CONSULTANT HARDWARE GROUP # M. Minimum Number of Required Plumbing Facilities: T.O. FTG. TOP OF FOOTING HARDWOOD **DESIGNED BY:** a) Water Closets - Required (m) O (f) Provided (m) (f) O
 b) Lavatories - Required (m) O (f) Provided (m) O (f) O T.O.P. TOP OF PLATE HEIGHT ES2 ENGINEERING STRUCTURAL SOLUTIONS **RECORD DRAWING DATE:** T/WALL TOP OF WALL HIGH POINT 442 North Main Street T. TREAD c) Bath Tubs or Showers: ___0 HORIZONTAL TYP. TYPICAL DETAIL REF. SYMB. Bountiful, Utah 84010 HOSE BIBB SIGNATURE: d) Drinking Fountains: ____0 Service Sinks: ___0 801-298-1118/Fax 801-298-1122 Prime Agency HOLLOW METAL UNF. UNFINISHED HOURS (FIRE RATING) © 2005 Spectrum Engineers, Inc. U.N.O. UNLESS NOTED OTHERWISE MECHANICAL CONSULTANT FOOTNOTES: INCH SHEET TITLE VAR. VARY OR VARIES INSIDE DIAMETER Spectrum Engineers 1) In case of conflict with the U.S. Department of Justice Federal Registers Parts I VERT. VERTICAL INSULATION through **∑** - ADA Guidelines and specific reference to the International Building INTR. ELEV. SYMB. 175 South Main Street, Suite 300 V.T.R. VENT THROUGH ROOF INTERIOR \bigcap Code Accessibility Chapters, the more restrictive requirement shall govern. Salt Lake City, Utah 84111 VCT VINYL COMPOSITION TILE COVER SHEET, INVERT ELEVATION 801-328-5151/Fax 801-328-5155 Additional Code Information shall be provided at the discretion of the Building WITH GENERAL INFO, Official for Complex Buildings. Including, but not limited to: W.A.S. WELDED ANCHOR STUD ELECTRICAL CONSULTANT J-BOX JUNCTION BOX WOOD & SHEET INDEX a) High Rise Requirements. DOOR/HDWR. SYMB. WATERPROOF Thomas & Kolkman Engineering b) Atriums WSCT. WAINSCOT 64 West 1700 South c) Performance Based Criteria. WITHOUT d) Means or Egress Analysis. Salt Lake City, Utah 84115 WORKING POINT e) Fire Assembly Locator Sheet. WATER RESISTANT 801-484-8161/Fax 801-385-3538 LAVATORY APPROVAL DOES NOT RELIEVE A/E OF DESIGN LIABILITY W.I. WROUGHT IRON f) Exterior and Interior Accessibility Route. LIGHT WINDOW SYMB. LOW POINT g) Fire Stopping, Including Tested Design Number.

· 	1	2	3 4	1 5	6	
		BASIS OF DESIGN 1. BUILDING CODE	CONCRETE 1. CODES AND STANDARDS. Comply with the following Codes:	MASONRY 1. CODES AND STANDARDS: Comply with ACI 530, "Building Code Requirements	D1 = 6d for #3 thru #8	Contract of the Contract of th
		2. BUILDING CLASSIFICATION CATEGORY II 3. GRAVITY DESIGN: DEAD LOADS Roofs 20 psf	A. ACI 301, "Specifications for Structural Concrete for Buildings". B. ACI 318, "Building Code Requirements for Reinforced Concrete". C. ACI 347, "Recommended Practice for Concrete Form Work".	for Masonry Structures". 2. MATERIALS: A. Lightweight concrete masonry units. ASTM C90, Grade N, Type 1.	D1 = 8d for #9 thru #11 (#11 bar Max. for MASONRY) D1 = 10d for #14 and #18 #ID1" #ID1" #ID4" #ID4" #ID4"	
		SNOW LOADS Snow load on flat roof, Pf 33 psf Importance factor: Is 1.0 Exposure factor: Ce 1.0	 MATERIALS shall conform to the following: A. Cement; ASTM C150, Type I, Portland Cement. B. Hard rock aggregates: ASTM C33 Lightweight aggregates: ASTM C330 	Minimum compressive strength = 1900 psi at 28 days (net area). B. Type "S" Portland cement-lime mortar. Minimum compressive strength = 1800 psi at 28 days. No additives. C. Grout shall conform to ASTM C476 with a minimum compressive	STD. HOOKS AND BENDS (TYP.)	Con Rock
		Thermal factor: Ct 1.1 4. WIND DESIGN: Basic wind speed 90 mph Importance factor: Iw 1.0	C. Water shall be potable. D. Air entrainment: ASTM C260 E. Fly ash: ASTM C618 F. Calcium chloride SHALL NOT be used.	strength of 2000 psi at 28 days. Use a fluid consistent grout, which may contain additional pea gravel if grout spaces are 4" or more in every direction. Limit fly ash to 20% of the total cement. D. Minimum compressive strength of masonry prism tests at 28 days:	D2 = 4d for #3 thru #5 D2 = 6d for #6 thru #8 * = #3 thru #5 * = #3 thru #5	175 South Main Street, Suite 300 Salt Lake City, Utah 84111 801-328-5151
E		Exposure	3. MIX DESIGNS:A. Place only one type of concrete at any given time.B. The maximum slump shall be 4" w/o plasticizer added.C. Use pea gravel and/or plasticizer in congested areas.	<pre>f'm = 1500 psi. 3. CONSTRUCTION: A. Store masonry under cover at the job site.</pre>	** = #6 thru #8 STIRRUP & TIE HOOKS (TYP.)	800-678-7077 FAX 801-328-5155 www.spectrum-engineers.com
		Spectral response accelerations: Ss & S1 1.41 & 0.55 Site class	D. Limit fly ash to 20% of the total cement. E. Concrete mixes shall conform to the following: TYPE OF 28 DAY MAX. MAX. AIR	B. Fully bed face shells. C. Tool Mortar joints concave. D. DO NOT use mortar for grout. E. DO NOT use any frozen materials. F. Use either low or high lift grouting procedures.	5 d2>d1 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	CONSULTANTS
		Basic Seismic-Force-Resisting System: Bearing walls special reinforced masonry walls Response modification factor: R 5 Seismic response coefficient: Cs 0.134	CONCRETE STRENGTH W/C DRY AGGREGATE MEMBER WEIGHT SIZE (psi) (ratio) (pcf) (inches) (%) Slabs on grade:	G. Consolidate grout by mechanical vibration during placing and reconsolidated after excess moisture has been absorbed but before workability is lost (45 minutes max.). H. Grout solid cells containing rebar, bolts, anchors, etc.	OFFSET SPLICE WIRE TIED LAP SPLICE MARK WALL TYPE LAP LENGTH L1 CONCRETE 30d2 for 40 GRADE 2'-0"	
		6. SOILS: Soil bearing pressure (Assuned) 1500 psf Lateral earth pressure	Interior 4000 0.47 150 1.5* 6 ± 1 *Well-graded Aggregates required (1.5"=> course >3/8", medium, and fines <#8) Follow ACI 302 for sand gradation.	I. Grout steel joist and steel beam pockets solid, UNO. J. Provide 1" of grout around bolts in side shells. K. DO NOT allow penetration through any beam, column, pier, or jamb without the EOR's approval. Otherwise, re-route penetrations at those	L1 CONCRETE 40d2 for 60 GRADE 2'-0" L1 MASONRY 48d2 2'-0" TYPICAL HOOKS & BENDS	
\exists		7. ABBREVIATIONS: EOR = Engineer of record. See professional stamp this page. UNO = Unless noted otherwise (E) = Existing condition	 4. CONSTRUCTION: A. Mechanically vibrate concrete during placement. B. Prior to placing concrete, check with trades to insure proper placement of openings, block outs, sleeves, curbs, conduits, bolts, 	locations. L. Prior to placing masonry, check with trades to insure proper placement of openings, block outs, sleeves, conduits, bolts, inserts, embeds, dowels, etc.	NO SCALE	
		(N) = New construction GENERAL	inserts, embeds, dowels, etc. Place anchor bolts and dowels prior to casting concrete, UNO. C. Add additional reinforcing too sides of floor and wall opening, equivalent to the bars cut by the opening with half to each side of the	4. WALLS: A. Use running bond. Build corners and intersections as an integral unit.		
		1. THE GENERAL CONTRACTOR SHALL: A. Be familiar with the contract documents and insure that subcontractors are familiar with their portion of the work. Submit a	opening or (2) $\#5$ bars, whichever is greater, UNO. Bars parallel to the principal reinforcing shall run full length of the span. End bars in the other direction with a standard hook. Add (2) $\#5 \times 5'-0"$ diagonal bars at every corner.	B. Dowel vertical reinforcing to the structure below and above with the same size bar and spacing, UNO. C. Place vertical reinforcing at the centerline of the wall unless each face (E.F.) is specified, UNO.		No.160916
		written request to the Arch/EOR for approval before proceeding with any changes. B. Verifies site conditions and dimensions at the site. If they differ from the contract documents, notify the Arch/EOR prior to	 5. SLABS ON GRADE (SOG): A. Minimum slab on grade requirements: 6 inches thickness. 6 inch layer of free-draining gravel base. 	D. Provide vertical reinforcing in grouted cells at corners and intersections. E. Terminate horizontal reinforcing at wall ends or openings with standard hooks or corner type bars. Provide corner bars of the same size		WILLMORE STATES
D		fabrication/construction of affected elements. Existing condition information on the drawings is based on best knowledge acquired during the design phase and may differ from actual conditions. Affected details may require redesign. C. Report to the Arch/EOR modifications made to the structure.	#4 bars at 18" o.c. both ways, UNO. Chair rebar for proper placement. B. See Architectural for exterior slabs on grade, UNO.	bar and spacing as the horizontal reinforcing at corners and intersections. F. Make horizontal bars continuous where concrete walls, columns, or pilasters interfaces. Provide a key between the masonry and concrete. Grout key solid.		D
		D. Be responsible for safety and protection on and around the job site and adjacent properties. 2. THE GENERAL CONTRACTOR SHALL COORDINATE:	REINFORCING STEEL 1. CODES AND STANDARDS. Comply with:	G. Construct bond beams at the top course and at floor and roof diaphragm interfaces. H. Construct penetrations thru walls as they are being laid. Add 2-#5 bars in grouted cells on all sides of opening which exceed 24 inches in		
		A. And verify locations, weights and sizes of mechanical units, equipment, etc. prior to the fabrication and erecting of structural supporting elements. Report sizes and locations that differ from those shown on the drawings to the Arch/EOR for review. Additional framing	a. CRSI "Manual of Standard Practice".b. ACI "Detailing Manual", ACI 315 (or SP-66).2. MATERIALS:	either direction, UNO. Extend vertical edge bars the full height of the wall between floor or roof support. Extend horizontal edge bars 24 inches beyond the opening edges. I. DO NOT place construction or expansion joints in beams, headers,		
		maybe required. B. Roof, floor, and wall openings required for mechanical, etc. which are not shown on the structural drawings with the Arch/EOR. C. Any structural situation not covered by the drawings with the	A. New stock deformed rebar: ASTM A615, Grade 60, except as noted. a. Field bent dowels: ASTM A615, Grade 40 or ASTM A706, Grade 60, Low-Alloy Steel. Reduce spacing of grade 40 dowels by 1/3. b. Welded rebar: ASTM A706, Grade 60, Low-Alloy Steel. B. Masonry joint wire: ASTM A82.	columns or supports, UNO. J. Reinforce masonry walls as follows, UNO. WALL VERTICAL HORIZONTAL THICKNESS REINFORCING REINFORCING		
4		Arch/EOR. D. Doors, windows, walls, elevations, slopes, stairs, curbs, drains, recesses, depressions, railings, waterproofing, finishes, chamfers, kerfs, pads, landscape walls, trenches in slabs, etc. with the structural work.	3. CONSTRUCTION:	8" 1-#6 at 48" o.c. 2-#4 at 48" o.c. U.N.O. Add ladder-type joint reinforcing of (2) 3/16 wires [(3) 3/16 wires with veneer] at 16" o.c. horizontally in all masonry walls. See plans,		_
		E. Inspections, testing, and structural observations as work proceeds. Notify the EOR 48 hours prior to any required structural observations.3. CONTRACT DOCUMENTS & DRAWINGS: A. These structural notes complement the specifications and the	B. Use rebar free of loose flaky rust, scale, grease, oil, dirt, and other materials, which affect or impair bond. C. Place rebar continuous in walls, beams, columns, slabs, footings, etc.	schedules, and details for other reinforcing. See plans, schedules, and details for other reinforcing.		SLCC
		drawings. B. Specific details, sections and notes shown on the drawings govern over these general notes and typical details. C. Contract documents take precedence over shop drawings, UNO.	D. Minimum lap splices (Inches): (Minimum lap 24") #3 #4 #5 #6 #7 #8 #9 #10 #11 Concrete: 24" 24" 33" 53" 66" 80" 96" 113"	5. BEAMS: A. Build beams as an integral part of their supports. No toothing or doweling is permitted. Provide masonry units with one opened end (No back-to-back end shells). Grout beams solid the full depth as shown in		REDWOOD ROAD
		D. Apply typical or similar details, sections and notes to similar situations on the drawings where specific details are not referenced. E. Drawings and details have been prepared to visually represent information provided in scaled form. However, DO NOT scale plans or	Masonry: See schedule 'Masonry - Minimum Bar Lap Lengths table'. E. Make cold bends. DO NOT use heat. Bends in the fabricator's shop, UNO. DO NOT unbend or rebend a previously bent bar.	the masonry beam schedule. B. Reinforcing in the masonry beam schedule is in addition to standard wall reinforcing. C. Place horizontal top bars in the top 4 inches of the beam and		CAMPUS STATE PROPERTY NO:
C		details for dimensional information.F. Refer to architectural drawings for dimensions.4. BUILDING CODE COMPLIANCE: Construction, inspection, materials, testing,	F. Minimum concrete cover: (securely position and anchor rebar prior to pour) Cast against and permanently exposed to earth 3" Exposed to earth or weather:	extend 72 bar diameters beyond the edge of the opening or terminate with a hook. Splice bars at mid-spans, UNO. D. Place horizontal bottom bars in the bottom 4 inches of the beam and extend 24 inches beyond the edge of the opening or terminate with a hook. Splice bars at supports, UNO.		C SLCC BUSINESS
		and workmanship shall conform to the requirements of the governing building code. 5. CONSTRUCTION SEQUENCE, SHORING, AND BRACING REQUIREMENTS: The general	#6 and larger	E. Hook vertical stirrups around bottom horizontal bars. Also hook them around the top horizontal bars or extend them into the wall above the beam a minimum of 48 bar diameters. Grout solid. F. Use the following masonry beam sizes in non-bearing masonry walls,		BUILDING CHILLER
		contractor is responsible for the method, means, and sequence of structural erection, UNO. He shall provide adequate temporary shoring or bracing for all structural elements until the entire structural system is completed. Design of shoring and bracing is by others at no additional cost to the owner.	Slabs-On-Grade (SOG) Center of slab, UNO G. In masonry, place and position rebar according to the structural drawings while laying units. Secure against displacement at intervals not	UNO: OPENING WIDTH BEAM WIDTH BEAM DEPTH HORIZ. REINF. Up to 4'-0" match wall 16" (2) #5 BOT.		REPLACEMENT
		6. OMISSIONS, CONFLICTS & DISCREPANCIES: A. Bring omissions, conflicts or discrepancies between the elements of the contract documents to the attention of the Arch/EOR before proceeding	to exceed the following: #4 and smaller	Up to 8'-0" match wall 24" (2) #5 T & B Up to 10'-0" match wall 32" (2) #5 T & B For wider openings consult with the EOR.		
		with work involved. B. In case of conflicts or discrepancies, follow the most stringent requirements as directed by the Arch/EOR.	H. DO NOT weld reinforcing unless specifically noted. Use E-90XX electrodes and ASTM A706 reinforcing. Comply with AWS requirements. I. Use epoxy coated reinforcing when specifically noted. Increase lap splice lengths by a factor of 1.2.	6. COLUMNS: Grout wall jambs (sides of openings) piers & columns solid the full height of member (floor to floor, etc.). Reinforce wall jambs with 2-#5 vertical bars for each grouted cell (one cell for each 4'-0" of span or portion thereof) with a #5 placed at each side face of the wall		SALT LAKE CITY, UTAH
		7. MISCELLANEOUS: A. During and after construction, builder and/owner shall keep loads on the structure within the limits of this design. See Basis of Design. B. Site observations by ES2's field representative shall neither be	STEEL DECK	jamb, UNO. STRUCTURAL STEEL		
		construed as inspection nor approval of construction 8. SUBMITTALS: A. Make submittals in a timely manner. ES2's review is for general	<pre>1. PRODUCT: A. See plans for deck size and gauge, etc. B. Manufacture deck and accessories from cold rolled steel conforming to ASTM A-446 Grade A (galvanized G-60) and conform to the Steel Deck</pre>	 CODES AND STANDARDS: Comply with: A. AISC "Specification for Structural Steel Buildings & Commentary". B. AISC "Code of Standard Practice" excluding sections 7.5.4, and 7.11.5. 		
		compliance only and is not intended as approval. Contractor is responsible for verifying sizes, dimensions and elevations on submittals as related to the contract documents. B. Submit the following items for review prior to proceeding with the work:	<pre>Institute and AISC Standards. 2. CONSTRUCTION: A. Install deck with a minimum of 4 spans, UNO. Where such layout is</pre>	 C. AWS "Structural Welding Code", exclude items conflicting with AISC. 2. MATERIALS SHALL CONFORM AS FOLLOWS: A. Wide Flange beams: ASTM A992 Fy = 50 ksi. 		
D		Concrete material Certifications & mix designs. Masonry material Certifications, grout & mortar designs. Shop Drawings: Reinforcing steel	<pre>impossible, decking must meet design criteria for simple span condition. B. Provide 2" minimum deck bearing and 4" lap at splice locations. Provide structural support (angles, etc.) around the perimeter of all deck openings. C. DO NOT bend or mar deck.</pre>	B. Other shapes & plates: C. High strength bolts: D. Other bolts: E. Welded anchors (studs) and deformed bar anchors (DBA's):		D
Б		Steel deck Structural steel Special loads on the structure. Roof, floor and wall openings not shown on the drawings.	D. Store decking off the ground with one end elevated. Cover with waterproof material and ventilate to avoid condensation. 3. DECK WELDING REQUIREMENTS:	manufacturer's specifications. DO NOT substitute reinforcing for DBA's.3. CONSTRUCTION: A. Fabricate in an approved fabricator's shop.		В
		Welding procedures and certifications. Veneer ties and anchors. C. Allow two weeks for the review of submittals by the EOR. D. Have EOR approved shop drawings & materials on site before	A. See plans for welding pattern. B. Weld to framing members with E60XX or E70XX electrodes. C. Use 3/4" diameter puddle welds. Penetrate all deck layers with weld metal and have proper fusion to the supporting steel.	B. Fabricate beams with incidental camber up, UNO. 4. BOLTED CONNECTIONS: A. Use 3/4" diameter bolts in Std. holes (bolt diameter + 1/16"), UNO.		<u></u>
		construction of those components begin. E. Substitutions are not allowed unless approved by the EOR. Submit requests for structural substitutions to the Arch/EOR.	D. Crimp side seams before welding side laps or use Verco's Punchlok system w/ connections @ 12" o.c. max. Engage all layers of the deck material. E. Overlap end laps at least 2" directly over a single steel support.	B. Steel-to-steel connections: Use ASTM A325 type "N" connections, UNO. C. Other connections: Use ASTM A307 bolts or better except for anchor rods, UNO.		3
		SPECIAL INSPECTION AND TESTING 1. INSPECTIONS: Provide special inspection by an independent agency in	Place welds 1" from the edge of the deck or more. F. Welds 3/8" x 1.5" long may replace 3/4" diameter welds.	D. Use hardened washers beneath the turned element of the bolt or nut. Use beveled hardened washers where the outer face of bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolts axis. At oversized holes, use hardened washers or plates at least 5/16" thick conforming to ASTM F436		MARK DATE DESCRIPTION
		accordance with IBC Chapter 17 and as outlined below: Anchor bolts and concrete & masonry embedments. Concrete: during pours, rebar placement, and taking of test specimens.	MASONRY - MINIMUM BAR LAP LENGTHS F'm = 1500psi	5/16" thick conforming to ASTM F436. E. Tighten bolts until all plies of the joint are in firm contact. Snug tight condition, UNO. F. Enlarge bolt holes by reaming. DO NOT torch cut.		ISSUE: CONSTRUCTION DOCS DATE: 30 NOVEMBER 2005
		Inspectors shall be ACI-II or ICC certified. Masonry: during placement, grouting, reinforcing, and taking of test specimens. Inspectors shall be ICC certified. Reinforcing steel: in concrete and masonry. Soils: bearing materials and placement of structural fill	2" 24" 31" 48" 98"* 133"* 186"* 235"* 2 1/2" 24" 25" 39" 78"* 106"* 149"* 188"* 3" 24" 24" 32" 65"* 89"* 124"* 157"* 3 1/2" 24" 24" 28" 56"* 76"* 106"* 134"* 100" 100" 100" 100" 100" 100" 100" 100	5. WELDED CONNECTIONS: A. Perform welding and cutting by AWS certified welders in accordance with ANSI/AWS D1.1 (latest edition). B. For typical shop & field welds, use filler metals with nominal 70		DFCM PROJECT NO: 05238660 PROJECT NO: 20050650 DRAWN BY: NT
		Soils: bearing materials and placement of structural fill. Steel Deck: (periodic) Steel to steel bolting: all high strength bolts. Welding: all field welds. Inspector shall be AWS-QC1 certified.	3 1/2" 24" 24" 28" 56" * 76" * 106" * 134" *	ksi tensile strength having: a. Matching material for multiple pass welds. b. A diffusible hydrogen limit of H16 or less. c. A CVN toughness of 20 ft-lbs at 0 deg. F.		CHECKED BY: KW DESIGNED BY: KW RECORD DRAWING DATE:
		2. TESTING: The owner will provide testing by qualified testing personnel for the following types of construction: Bolting: installation and correct torque and/or tension.	 THE BAR COVER DISTANCE, K, SHALL BE TAKEN AS THE LEAST DIMENSION AS SHOWN HERE. FOR BAR COVER DISTANCES, K, NOT SHOWN CONTACT E.O.R. MINIMUM YIELD STRENTGH OF REINFORCEMENT; fy = 60,000psi. 	C. Use pre-qualified procedures. D. Weld intersecting steel shapes together, which are not connected with bolts, with all-around fillet welds, UNO. E. Weld studs and DBA's according to Manufacturer's specs.		SIGNATURE: © 2005 Spectrum Engineers, Inc.
A		Concrete: strength, slump, air, and temperature. Masonry: strength of mortar, grout, block, and prisms. Soils: compaction. Welding: type, size, length, and quality of shop and all field welds	5. MECHANICALLY SPLICE BARS GREATER THAN #9. 6. * NOT ALLOWED AS VERT. REINF IN LOW LIFT GROUTED WALLS. K K K K K K K K K K K K K	F. Wherever possible use shop welds. The contractor shall coordinate field and shop welds between shop fabrication and the steel erector. G. Remove slag from welds.		A SHEET TITLE
		by approved methods.3. THE CONTRACTOR SHALL: A. Coordinate testing. DO NOT proceed with subsequent work until				GENERAL STRUCTURAL
		<pre>inspections and testing has been approved. B. Copy inspection reports/testing results to the Arch/EOR and owner before work proceeds. C. Correct deficient work at no additional cost to the owner.</pre>				NOTES
						SG-101
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